ANY MODIFICATIONS TO THESE PLANS OR RE-USE OF THESE PLANS MUST BE APPROVED BY AND SIGNED BY THE ARCHITECT.

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SYMBOLS AND ABBREV.

ALT	ALTERNATE/ALTERNATIVE
ACI	AMERICAN CONCRETE INSTITUTE
AFF	ABOVE FINISHED FLOOR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AISI	AMERICAN IRON AND STEEL INSTITUTE
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
AWS	AMERICAN WELDING SOCIETY
AB	ANCHOR BOLTS
ARCH	ARCHITECTURE/ARCHITECTURAL
ASTM	AMERICAN SOCIETY OF TESTING MATERIALS
AWS	AMERICAN WELDING SOCIETY
BB	BOND BEAM
BFB	BOTTOM FLANGE BRACE
BP	BASE PLATE/BEARING PLATE
BRG	BEARING
BM	BEAM
BLK	BLOCK
B/	BOTTOM OF
BLDG	BUILDING
CANT	CANTILEVER
CL	CENTERLINE
CLR	CLEAR/CLEARANCE
COL	COLUMN
CONC	CONCRETE
CB	CONCRETE BEAM
CC	CONCRETE COLUMN
CMU	CONCRETE MASONRY UNIT
CONT	CONTINUOUS
CONNX	CONNECTION
CONST	CONSTRUCTION
CS J	CONSTRUCTION JOINT
C J	CONTRACTION JOINT / CONTROL JOINT
DET	DETAIL
DEPT	DEPARTMENT
DBA	DEFORMED BAR ANCHOR
DFT	DRY FILM THICKNESS
DIA	DIAMETER
DIM	DIMENSION
DIST	DISTANCE
DN	DOWN
DR	DRAIN
DWG	DRAWING
EA	EACH
EE	EACH END
EF	EACH FACE
EJ	EXPANSION JOINT
EL	ELEVATION
EMB	EMBEDMENT
ENGR	ENGINEER
EOS	EDGE OF SLAB
EQ	EQUAL
ES	EACH SIDE
EW	EACH WAY
EXIST	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
FBC	FLORIDA BUILDING CODE
FL	FULL LENGTH WELD. WELD ENTIRE DIST.
F V	FIELD VERIFY
F F	FINISHED FLOOR
FLR	FLOOR
F D	FLOOR DRAIN
FTG	FOOTING
GA	GAGE/GAUGE
GALV	GALVANIZED
GC	GENERAL CONTRACTOR
GLU-LAM	GLUE LAMINATED
HAS	HEADED ANCHOR STUD
HC	HOLLOW CORE
HK	HOOK
HORIZ	HORIZONTAL
HP	HIGH POINT
HSS	HOLLOW STRUCTURAL SECTION
HT	HEIGHT
ID	INSIDE DIAMETER
IF	INSIDE FACE
INT	INTERIOR
JT	JOINT
JST	JOIST

K	KIP = 1000 LB
KO	KNOCK OUT
LGTH	LENGTH
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
MANUF	MANUFACTURE/MANUFACTURER
M B	MASONRY BEAM
MATL	MATERIAL
MAX	MAXIMUM
MECH	MECHANICAL
MEZZ	MEZZANINNE
MIN	MINIMUM
MISC	MISCELLANEOUS
MO	MASONRY OPENING
MTL	METAL
N I C	NOT IN CONTRACT
NOM	NOMINAL
N T S	NOT TO SCALE
N W T	NORMAL WEIGH TOPPING
O C	ON CENTER
OPNG	OPENING
OPP	OPPOSITE
PAF	POWER ACTUATED FASTENER
PL	PLATE
PLY	PLYWOOD
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PC	PRECAST CONCRETE
PRE-ENG	PRE-ENGINEERED
PREFAB	PREFABRICATED
PROJ	PROJECTION
PT	PRESSURE TREATED
PW	PANEL WIDTH
REF	REFERENCE
REINF	REINFORCING
R C P	REINFORCED CONCRETE PIPE
REQD	REQUIRED
R W	RETAINING WALL
R D	ROOF DRAIN
SCHED	SCHEDULE
SIM	SIMILAR
SPC	SPACE/SPACES
SPECS	SPECIFICATIONS
SPF	SPRUCE PINE FUR
SQ	SQUARE
S A	STUD ANCHOR
S S	STAINLESS STEEL
STD	STANDARD
STL	STEEL
STRUC	STRUCTURAL
SYM	SYMMETRICAL
SF	STEPPED FOOTING
SYP	SOUTHERN YELLOW PINE
THK	THICK
THD	THREAD/THREADED
TB	TIE BEAM
T & B	TOP AND BOTTOM
T & G	TONGUE AND GROOVE
T.O.C.	TOP OF CONCRETE
T.O.S.	TOP OF STEEL
TRANS	TRANSVERSE
TYP	TYPICAL
TS	TUBE STEEL
T/	TOP OF
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
VOL	VOLUME
WF	WALL FOOTING
WPF	WATERPROOF
WWF	WELDED WIRE FABRIC
WH	WEEP HOLE
WT	WEIGHT
WF	WIDE FLANGE
W /	WITH
W / O	WITHOUT
WD	WOOD
WP	WORKING POINT

GENERAL NOTES

G1 THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS AT THE SITE AND SHALL NOTIFY THE ARCHITECT OF DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH THE WORK.

G2 THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE COMPLETE DESIGN OF THE STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKMEN, OR OTHER PERSONS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE BUT NOT BE LIMITED TO BRACING, SHORING FOR EARTH BANKS, FORMS, SCAFFOLDING, PLANKING, SAFETY NETS, SUPPORT AND BRACING FOR CRANES AND GIN POLES.

G3 THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL MEMBERS AS REQUIRED FOR STRUCTURAL STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER OF ANY CONDITION WHICH, IN HIS OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS IN THE STRUCTURE.

G4 CONSTRUCTION MATERIALS SHALL NOT BE STACKED ON FLOORS OR ROOFS IN EXCESS OF THE DESIGN LIVE LOADS WHICH ARE INDICATED IN THE GENERAL NOTES. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO INSURE THAT THE SUBCONTRACTORS ARE INFORMED AND DO NOT VIOLATE THIS IMPORTANT REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON FLOORS OR ROOFS.

G5 PLANS, SECTIONS AND DETAILS ARE NOT TO BE SCALED FOR DETERMINATION OF QUANTITIES, LENGTHS, OR FIT OF MATERIALS.

G6 SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR MISCELLANEOUS STEEL ITEMS NOT SHOWN HEREON.

G7 COORDINATE SIZES AND LOCATIONS OF OPENINGS IN FLOORS AND ROOF WITH ARCHITECTURAL, MECHANICAL AND ELECTRICAL REQUIREMENTS.

G8 FOR ACTUAL DATUM ELEVATION OF FIRST FLOOR REFERENCE EL. 100'-0", SEE SITE PLAN.

G9 SUBMIT WRITTEN REQUEST TO THE ARCHITECT FOR APPROVAL OF ANY PROPOSED CHANGE TO THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. SPLICING, CUTTING, NOTCHING OR OTHER ALTERATIONS TO STRUCTURAL MEMBERS ARE NOT PERMITTED WITHOUT WRITTEN AUTHORIZATION OF THE STRUCTURAL ENGINEER. ANY UNAUTHORIZED DEVIATION FROM THE CONTRACT DOCUMENTS, AND CORRECTION THEREOF, IS THE RESPONSIBILITY OF THE CONTRACTOR.

G10 THE ENGINEER DOES NOT HAVE CONTROL OR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK, FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

G11 PERIODIC SITE OBSERVATION BY FIELD REPRESENTATIVES OF SOUTHARD ENGINEERING, INC. IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR IS PROCEEDING IN ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINOUS TO CHECK THE QUALITY OR QUANTITY OF THE WORK, BUT RATHER PERIODIC IN AN EFFORT TO GUARD THE OWNER AGAINST DEFECTS OR DEFICIENCIES IN THE WORK OF THE CONTRACTOR.

G12 SOUTHARD ENGINEERING WILL PROVIDE OBSERVATION REPORTS ITEMIZING STRUCTURAL DEFICIENCIES. THE OBSERVATION REPORTS ARE MADE TO OBSERVE PROGRESS OF THE PROJECT AND TO MAKE SURE THE BASIC INTENT OF THE CONSTRUCTION DRAWINGS ARE BEING FOLLOWED. THUS, THEY ARE NOT AN EXHAUSTIVE INSPECTION, BUT A BRIEF OBSERVATION TO HELP AVOID SYSTEMIC PROBLEMS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO CORRECT ALL NOTED DEFICIENCIES. THE CONTRACTOR SHALL HAVE THE FIELD INSPECTORS OBSERVE AND VERIFY THAT ALL NOTED DEFICIENCIES HAVE BEEN CORRECTED. SOUTHARD ENGINEERING WILL NOT BE LIABLE FOR STRUCTURAL DEFICIENCIES DUE TO CONTRACTOR'S DISREGARD AND/OR INABILITY TO CORRECT THE NOTED DEFICIENCIES.

DESIGN CRITERIA

D1 STRUCTURAL WORK SHALL BE IN ACCORDANCE WITH THE 2007 FLORIDA BUILDING CODE (FBC)

D2 THE MOST STRINGENT REQUIREMENTS APPLY IN CASE OF CONFLICT BETWEEN SPECIFICATIONS, STANDARDS, CODES AND DRAWINGS.

D3 DESIGN DATA

1. ROOF LIVE LOADS — 20 PSF
2. ROOF DEAD LOADS — 20 PSF
   TRUSS TOP CHORD DEAD LOAD — 10 PSF
   TRUSS BOT. CHORD DEAD LOAD — 10 PSF
3. FLOOR LIVE LOADS — VARY, SEE PLANS
   CORRIDORS, UPPER FLOOR .... 100 PSF
   SLAB ON GRADE .... 100 PSF
   OFFICES .... 50 PSF
   STAIRS .... 100 PSF

   AN ADDITIONAL DEAD LOAD ALLOWANCE OF 15 PSF FOR PARTITIONS HAS BEEN PROVIDED FOR FLOOR AREAS DESIGNED FOR LIVE LOADS OF LESS THAN 80 PSF.

   STAIR TREADS (ON CENTER OF TREADS)......300 LBS.

DESIGN CRITERIA (CONT'D)

4. FLOOR DEAD LOADS — 18 PSF
5. WIND VELOCITY (ASCE 7-05) — 110 MPH EXPOSURE C
6. IMPORTANCE FACTOR — 1.0
7. INTERNAL PRESSURE COEFFICIENT — +/- 0.18
8. ALLOWABLE SOIL PRESSURE — 2500 PSF
9. WINDOW WIND PRESSURE — 39 PSF

REQUIRED SHOP DRAWING SUBMITTALS

SD1 APPROVAL OF SHOP DRAWINGS DOES NOT INDICATE ACCEPTANCE OF DEVIATIONS FROM CONTRACT DOCUMENTS, UNLESS ACCEPTED BY THE ENGINEER IN WRITING PRIOR TO SUBMISSION OF SHOP DRAWINGS. CONFLICTS RESULTING FROM SUCH DEVIATIONS, CONFLICTS BETWEEN THIS WORK AND THE WORK OF OTHER TRADES DUE TO SUCH DEVIATION, AND DIMENSIONAL CONFLICTS AS A RESULT OF SUCH DEVIATIONS SHALL BE DEEMED THE CONTRACTOR'S RESPONSIBILITY.

SD2 ANY CHANGES TO THE DETAILS SHOWN IN THESE CONTRACT DOCUMENTS SHALL BE SUBMITTED IN WRITING BY R.F.I. AND APPROVED BY THE ARCHITECT AND ENGINEER PRIOR TO SUBMITTING SHOP DRAWINGS. ALL SUCH CHANGES SHALL BE "CLOUDED" ON THE SHOP DRAWINGS AND REFERENCED TO THE PROPER R.F.I.

SD3 SUBMITTALS SHALL CONFORM TO THE REQUIREMENTS OF THE CONTRACT DRAWINGS. NON-CONFORMING SUBMITTALS WILL BE RETURNED WITHOUT REVIEW.
1. SHOP DRAWINGS SHALL BE CHECKED AND MARKED "APPROVED" BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL TO ARCHITECT.
2. SHOP DRAWINGS SHALL NOT CONTAIN REPRODUCTIONS OF THE CONTRACT DRAWINGS.
3. SUBMIT FOR ENGINEER'S REVIEW THE SHOP DRAWINGS FOR THE FOLLOWING ITEMS.

SD4 ONCE SHOP DRAWINGS ARE REVIEWED BY THE ENGINEER THE GC MAY USE THE MATERIAL FOR CONSTRUCTION. THE G.C. SHALL ALLOW FOR 3 WEEKS OF REVIEW TIME IN THE CONST. SCHEDULE.

SD5 THE FOLLOWING SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL.
1. STEEL STAIR SHOP DRAWINGS
2. REINFORCING STEEL (FOUNDATIONS, BEAMS, & CMU WALLS.
3. STEEL DECK (ROOF AND FLOOR)
4. CONCRETE MIX DESIGNS (FOUNDATIONS, SLABS, AND CMU GROUT)
5. STRUCTURAL STEEL
6. TIMBER ROOF AND FLOOR TRUSSES
7. CMU BLOCK TYPE AND MORTAR MIX DESIGNS.
8. SHELF ANGLES AND MISC. STEEL.

SPECIALTY ENGINEERING REQUIREMENTS

SE1 DELEGATED ENGINEER REQUIREMENTS: THE FLORIDA BOARD OF PROFESSIONAL ENGINEERS HAS ISSUED STATEMENTS ON RESPONSIBILITIES OF PROFESSIONAL ENGINEERS, PURSUANT TO CHAPTERS 61G15-30 AND 61G15-31 OF THE FLORIDA ADMINISTRATIVE CODE. CERTAIN COMPONENTS OF THE STRUCTURE REQUIRE THE WORK OF DELEGATED ENGINEERS FOR THE DESIGN OF THOSE COMPONENTS. ALL RELEVANT PROCEDURES PRESENTED IN THE FLORIDA ADMINISTRATIVE CODE SHALL APPLY TO THIS PROJECT.

SE2 STAIRS SHALL BE DESIGNED BY THE FABRICATORS SPECIALTY ENGINEER AND SHALL INCLUDE STRINGERS, TREADS, HAND RAILINGS, PLATFORMS (AS REQ'D), PAN INSERTS, AND MISCELLANEOUS SUPPORTS AND CONNECTIONS. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. MINIMUM DESIGN LIVE LOAD SHALL BE 100 PSF. STAIR TREADS SHALL BE DESIGNED FOR A MIN. 300# PT. LOAD IN THE CENTER.

SE3 HANDRAILS, POSTS, AND SUPPORT CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR'S SPECIALTY ENGIEER. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED AND SEALED BY A ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADINGS SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE.

SE4 EXTERIOR CURTAIN WALLS SHALL BE DESIGNED BY THE VENDOR'S SPECIALTY ENGINEER AND SHALL INCLUDE FRAME, GLASS GLAZING,, AND CONNECTIONS. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADINGS SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE. (SEE DESIGN CRITERIA FOR APPLICABLE BUILDING CODE). VENDOR SHALL PROVIDE WINDOW WALL REACTIONS TO THE ARCHITECT.

SE5 FLAG POLES AND/OR SITE LIGHTING POLES SHALL BE DESIGNED BY THE POLE VENDOR'S SPECIALTY ENGINEER AND SHALL INCLUDE POLES, FOUNDATIONS AND CONECTIONS. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE POJECT LOCATION. MINIMUM DESIGN LOADS SHALL CONFORM TO ANSI/NAAM FP100 "SPECIFICATIONS FOR THE DESIGN LOADS OF METAL FLAGPOLES".

SE6 EXTERIOR LIGHTGAGE STEEL FRAMING, INCLUDING BUT NOT LIMITED TO: WALLS, FASCIAS, AND SOFFITS SHALL BE DESIGNED BY A SPECIALTY ENGINEER. STRUCTURAL ELEMENTS HAVE BEEN PROVIDED FOR THE ATTACHMENT OF WALL FRAMING. THE STUD WALL SYSTEM SUPPLIER SHALL DESIGN AND DETAIL ALL CONNECTIONS TO THESE ELEMENTS. ANY FURTHER ELEMENTS REQ'D FOR THE SUPPORT OF STUD WALLS SHALL BE DESIGNED AND SUPPLIED AS PART OF THE STUD WALL SYSTEM. SHOP DRAWINGS SHALL BE SUMITTED FOR REVIEW AND MUST BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADINGS SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE. (SEE DESIGN CRITERIA FOR THE APPLICABLE BUILDING CODE).

SE7 AWNINGS SHALL BE DESIGNED BY THE FABRICATOR'S SPECIALTY ENGINEER AND SHALL INCLUDE FRAME, COVERING, AND CONNECTIONS. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED AND SEALED BY A STRUCTURAL ENGINEER IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADINGS SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE. (SEE DESIGN CRITERIAL FOR THE APPLICABLE BUILDING CODE).

FOUNDATION NOTES

F1. ANY FILL REQUIRED TO BACKFILL EXCAVATED AREA OR ACHIEVE FINISHED GRADE IN STRUCTURAL AREAS SHALL BE INORGANIC, NON-PLASTIC GRANULAR SOIL (CLEAN SANDS). THE FILL SHALL BE PLACED IN LEVEL LIFTS NOT TO EXCEED 12 INCHES LOOSE THICKNESS AND COMPACTED TO A MINIMUM OF 95% OF THE SOIL'S MODIFIED PROCTOR MAXIMUM DRY DENSITY AS DETERMINED BY ASTM SPECIFICATION D-1557. IN-PLACE DENSITY TESTS SHALL BE PERFORMED ON EACH LIFT BY AN EXPERIENCED ENGINEERING TECHNICIAN TO VERIFY THAT THE REQUIRED DEGREE OF COMPACTION HAS BEEN ACHIEVED. A SOIL COMPACTION TEST SHALL BE PERFORMED IN EVERY SPREAD FOOTING PAD SUB-GRADE.

F2. REMOVE FREE WATER FROM EXCAVATIONS BEFORE PLACING CONCRETE.

F3 CONTRACTOR ALONG WITH GEOTECHNICAL FIELD REPRESENTATIVE SHALL ENSURE THAT CLAYEY SOILS OR EXPANSIVE SOILS ARE NOT PRESENT.

F4 CONTRACTOR IN CONJUCTION WITH GEOTECHINICAL FIELD REPRESENTATIVE, SHALL DETERMINE IF ANY SOILS OR UNSUITABLE CONDITIONS ARE DISCOVERED DURING EXCAVATION WHICH WOULD PREVENT ATTAINMENT OF THE DESIGN SOIL PRESSURE RECOMMNDED BY THE SOILS REPORT.

F5 ALL SOIL PREPARATION SHALL CONFORM TO THE RECOMMENDATIONS CONTAINED IN THE SOILS REPORT FOR THE PROJECT AND SPECIFICATIONS.

F6 FOUNDATION DESIGN IS BASED ON SOILS REPORT AND RECOMMENDATIONS BY CAL-TECH TESTING, INC. (PROJECT NUMBER 10-00437-01) AND DATED NOVEMBER 23, 2010. THE GENERAL CONTRACTOR MAY REVIEW A COPY OF THE GEOTECHNICAL REPORT AT THE OFFICE OF THE ARCHITECT.

SLAB ON GRADE

SOG1 UNLESS NOTED OTHERWISE IN THE GEOTECHNICAL REPORT, COMPACT INTERIOR FILL TO 95% OF MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D1557). SOIL COMPACTION SHALL BE FIELD-CONTROLLED BY A REPRESENTATIVE TECHNICIAN OF A QUALIFIED LABORATORY. EACH LAYER OF FILL SHALL NOT EXCEED 12" THICK AND SHALL BE COMPACTED PRIOR TO PLACEMENT OF NEXT LAYER. SLAB ON GRADE SHALL BE CAST OVER A VAPOR BARRIER.

SOG2 MAXIMUM SPACING OF CONTROL JOINTS SHALL BE AS SET IN THE TABLE BELOW, OR AS NOTED ON PLANS. THE MORE STRINGENT SHALL APPLY. PATTERNS SHALL BE APPROXIMATELY SQUARE WITH A RATIO OF LONG SIDE TO SHORT SIDE NOT EXCEEDING 1.5 TO 1.

SLAB THICKNESS (IN)	* 3/4" OR LARGER AGGREGATE SPACING (FT)
4	12
5	13
6	15
7	18
8	20
9	23
10	25

* MIX DESIGNS CONTAINING AGGREGATE LESS THAN 3/4" ARE NOT ACCEPTABLE

CUT SLAB WITHIN 12 HOURS

SOG3 GENERAL CONTRACTOR SHALL COORDINATE EXACT LOCATION OF SAW JOINTS AND CJ's WITH ARCHITECTURAL FLOOR FINISHES TO ENSURE SLAB JOINTS DO NOT READ THROUGH.

CONCRETE AND REINFORCING

C1 A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD TESTING INCLUDING SLUMP TESTS AND CYLINDER BREAKS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.

C2 CONCRETE WORK SHALL CONFORM TO ACI SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS ACI 301 (LATEST EDITION) AND BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-02).

C3 ALL CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES.

LOCATION	28 DAY STRENGTH	SLUMP	COURSE AGGREGATE	
			MIN.	MAX.
FOUNDATIONS	3000 PSI	4" +/- 1"	3/4"	1 1/2"
SLAB ON GRADE: 4" THICK	3000 PSI	4" +/- 1"	3/4"	1 1/2"
SLAB ON GRADE: 5" & THICKER	4000 PSI (MINIMUM FLEXURAL STRENGTH = 550 PSI.)	4" +/- 1" SEE NOTE 1	3/4"	1 1/2"
TIE BEAMS AND TIE COLUMNS	3000 PSI	4" +/- 1"	3/8"	3/4"
BEAMS AND COLUMNS	4000 PSI	4" +/- 1"	3/4"	1"
FILLED CELLS, PRECAST LINTELS & BOND BEAM GROUT (ASTM C476) - SEE NOTE 2	3000 PSI	8" TO 11"	SAND	3/8"

NOTES:
1. SLUMP FOR RAMPS AND SLOPING SURFACES SHALL NOT EXCEED 4".
2. SEE MASONRY NOTE M10 FOR TESTING REQUIREMENTS OF GROUT TO BE USED TO FILL CORES OF CMU.

C4 CONCRETE MIX DESIGN SUBMITTALS.
1. EACH MIX DESIGN SHALL BE LABELED TO INDICATE THE AREA IN WHICH THE CONCRETE IS TO BE PLACED (I.E. FOUNDATIONS, SLAB-ON-GRADE, COLUMNS, ETC). FAILURE TO DO SO WILL CAUSE DELAY AND/OR REJECTION OF SUBMITTALS.
2. PROPOSED MIX DESIGN SHALL BE IN ACCORDANCE WITH METHOD 1 OR METHOD 2 OF ACI 301. PROVIDE SUPPORTING DATA IN TABULAR FORM FOR EACH SEPERATE PROPOSED MIX.
3. SUBMIT CONCRETE MIX DESIGN FOR EACH PROPOSED CLASS OF CONCRETE.

C5 NO CALCIUM CHLORIDE SHALL BE USED IN MIX DESIGNS.

C6 MAXIMUM W/C RATIO OF 0.55 FOR FOOTINGS AND 0.50 FOR OTHER CONCRETE. CMU GROUT SHALL HAVE W/C RATIO OF 0.60 OR HIGHER. MAX W/C RATIO FOR TILT-UP PANELS SHALL BE 0.45.

C7 REINFORCING BARS SHALL CONFORM TO ASTM A-615, GRADE 60.

C8 WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. LAP MINIMUM DISTANCE OF ONE CROSS WIRE SPACING PLUS 2 INCHES.

C9 SPLICE REINFORCING ONLY WHERE SHOWN ON THE DRAWINGS. WHERE CONTINUOUS REINFORCING IS CALLED OUT, SUCH REINFORCING MAY BE SPLICED WHERE APPROVED BY THE ENGINEER. WHERE SPLICE LENGTHS ARE NOT SPECIFIED, USE 48 BAR DIAMETERS IN MASONRY AND 48 BAR DIAMETERS IN CONCRETE.

C10 PROVIDE CONCRETE COVER OVER REINFORCEMENT AS FOLLOWS, UNLESS OTHERWISE NOTED:
- BEAMS (OVER STIRRUPS) — 1-1/2" (#5 BARS AND SMALLER); 2" (#6 THROUGH #18)
- COLUMNS (OVER TIES) — 1-1/2" (#5 BARS AND SMALLER); 2" (#6 THROUGH #18)
- FOOTINGS — 3"
- SLABS — 1-1/2" FROM TOP

C11 AT CHANGES IN DIRECTION OF CONCRETE WALLS AND BEAMS, PROVIDE CORNER BARS OF SAME SIZE AND SPACING AS HORIZONTAL STEEL.

C12 PROVIDE STANDARD HOOKS FOR ALL TOP REINFORCING BARS AT DISCONTINUOUS ENDS. HOOKS MAY BE TILTED FROM VERTICAL TO OBTAIN PROPER CONCRETE COVER.

C13 GROUT UNDER BEARING PLATES SHALL BE NON-METALLIC, NON-SHRINK TYPE WITH A COMPRESSIVE STRENGTH OF AT LEAST 6000 PSI IN SEVEN DAYS. VIBROPRUF #11, BY LAMBERT CORPORATION, OR ACCEPTED SUBSTITUTE.

C14 ALL FORMWORK SHALL BE DESIGNED, ERECTED, SUPPORTED, BRACED, AND MAINTAINED ACCORDING TO ACI 347, RECOMMENDED STANDARD PRACTICE FOR CONCRETE FORMWORK.

C15 RESPONSIBILITY: THE DESIGN, CONSTRUCTION, AND SAFETY OF ALL FORMWORK SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR.

C16 ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED WHERE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS.

STRUCTURAL STEEL NOTES

S1 STEEL WORK SHALL CONFORM TO THE AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS- ALLOWABLE STRESS DESIGN AND AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES.

S2 MATERIAL SHALL CONFORM TO THE FOLLOWING, EXCEPT AS NOTED:
ROLLED SHAPES, PLATES AND BARS: ASTM A36, EXCEPT,
WIDE FLANGE SECTIONS: ASTM A992, GRADE 50. fy=50ksi.
MACHINE BOLTS: ASTM A307.
PIPE COLUMNS: ASTM A53, GRADE B. fy=36ksi
STRUCTURAL STEEL TUBING, ASTM A500, GRADE B. fy=46ksi
HIGH STRENGTH BOLTS: ASTM A 325 U.N.O.
ANCHOR RODS: ASTM FI554 GRADE 55
HEADED ANCHOR STUDS: ASTM A108 (ULT TENSILE STR = 60,000 psi)

S3 CONNECTIONS:
1. UNLESS OTHERWISE NOTED, BOLTS SHALL BE HIGH-STRENGTH, BEARING TYPE WITH THREADS INCLUDED IN SHEAR PLANES. TIGHTEN BY THE "TURN-OF-THE-NUT" METHOD (SNUG-TIGHT PLUS 1/2 TURN).
2. WELDING ELECTRODES FOR ALL STEEL SHALL BE E70XX. RETURN FILLET WELDS FOR FRAMED CONNECTIONS 1/2" AT EACH END.
3. SHOP CONNECTIONS SHALL BE WELDED OR BOLTED.
4. FIELD CONNECTIONS SHALL BE MADE WITH 3/4" BOLTS, EXCEPT AS NOTED OTHERWISE.
5. ALL CAP PLATES AND BASE PLATES SHALL BE CONTINUOUSLY WELDED TO COLUMNS W/ MAX WELD SIZE UNO.

S4 HIGH-STRENGTH FIELD-BOLTED CONNECTIONS SHALL BE INSTALLED, TIGHTENED, TESTED, AND INSPECTED ACCORDING TO "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" BY RESEARCH COUNCIL ON STRUCTURAL CONNECTMIONS (RCSC). CONNECTIONS SHALL NOT BE CLASSIFIED AS SLIP-CRITICAL (SC) UNLESS INDICATED ON PLANS AS SUCH. "SNUG-TIGHT", AS DEFINED IN THE SPECIFICATION, IS SUFFICIENT FOR ALL BOLTED CONNECTIONS UNLESS THE BOLTS IN SUCH A CONNECTION ARE INDICATED AS SLIP-CRITICAL (SC). SLIP-CRITICAL BOLTS MUST BE FULLY TENSIONED PER SPECIFICATION.

S5 BRACE AND MAINTAIN ALL STEEL IN ALIGNMENT UNTIL OTHER PARTS OF CONSTRUCTION NECESSARY FOR PERMANENT SUPPORT ARE COMPLETED. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING TEMPORARY SHORING AS REQUIRED FOR THE STABILITY OF THE STEEL FRAME UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN COMPLETED AND BUILDING IS ENCLOSED.

S6 ALL WELDING IN THE SHOP OR IN THE FIELD SHALL BE PERFORMED BY CERTIFIED WELDERS ONLY. CERTIFICATION DOCUMENT SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR HIS REVIEW. ALL WELDS SHALL BE PRE-QUALIFIED PER AWS D1.1-94. WELDED SPLICES OF ROLLED SHAPES MADE IN THE SHOP ARE ACCEPTABLE PROVIDED RADIOGRAPHED NDT EXAMINATION RESULTS ARE IN ACCORDANCE WITH AWS ACCEPTANCE STANDARDS AND WRITTEN REPORTS VERIFYING SUCH RESULTS ARE SUBMITTED TO THE STRUCTURAL ENGINEER FOR HIS APPROVAL. MINIMUM FILLET WELDS SHALL BE 3/16 UNLESS OTHERWISE SHOWN ON THE DRAWINGS.

S7 THE STEEL STRUCTURE IS DESIGNED FOR STABILITY IN ITS COMPLETED CONDITION PER THE DRAWINGS, SPECIFICATIONS AND THESE NOTES. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY BRACING, GUYING AND OTHER MEANS OF SUPPORT DURING CONSTRUCTION SUFFICIENT TO WITHSTAND WEATHER CONDITIONS AND MEET ALL APPLICABLE SAFETY REQUIREMENTS DURING CONSTRUCTION.

S8 DETAILING OF STRUCTURAL STEEL AND CONNECTIONS SHALL BE SHOWN ON SHOP AND ERECTION DRAWINGS PREPARED BY THE FABRICATOR FOR THE STRUCTURAL ENGINEER'S REVIEW, PRIOR TO FABRICATION.

S9 ALL STEEL SHALL RECEIVE A SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) WHERE EXPOSED TO VIEW. ALL OTHER AREAS, INCLUDING THOSE WHICH WILL RECEIVE SPRAY-ON-FIRE PROTECTION, OR WHERE HEADED STUDS ARE TO BE WELDED, SHALL NOT BE

S10 HOT DIP GALVANIZE AFTER FABRICATION ALL STRUCTURAL STEEL ITEMS AND THEIR CONNECTIONS PERMANENTLY EXPOSED TO EARTH AND/OR TO WEATHER, GALVANIZING SHALL BE PER ASTM A123 FOR MEMBERS AND ASTM A153 FOR CONNECTION ELEMENTS. (SEE DRAWINGS FOR OTHER STRUCTURAL ITEMS TO BE HOT DIP GALVANIZED).

S11 PROVIDE CURB ANGLES 3X3X1/4 TO SUPPORT ROOF DECK AT OPENINGS UNO.

CONCRETE MASONRY UNITS NOTES

M1 MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530-05/ASCE 5-05/TMS 402-05) AND "SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1/ASCE 6/TMS 602)." REFERRED TO HEREAFTER AS (SMS).

M2 THE COMPRESSIVE STRENGTH OF MASONRY (f'm) SHALL BE AT LEAST 1500 psi. THIS SHALL BE DETERMINED BEFORE CONSTRUCTION.

M3 USE THE UNIT STRENGTH METHOD TO DETERMINE f'm. DETERMINE f'm BASED ON STRENGTH OF UNIT AND TYPE OF MORTAR SPECIFIED. UNITS SHALL CONFORM TO ASTM C90, TYPE II, NORMAL WT. AND SHALL BE TESTED IN ACCORDANCE WITH ASTM C140 THICKNESS OF BED JOINTS SHALL NOT EXCEED 5/8 IN. LAID IN RUNNING BOND ONLY.

M4 THE G.C. SHAL SUBMIT MIX DESIGNS AND TEST RESULTS FOR MORTAR AND GROUT BEFORE CONSTRUCTION BEGINS.

M5 MORTAR SHALL COMPLY WITH ASTM C270, TYPE M FOR RETAINING WALLS AND WALLS BELOW GRADE, TYPE S FOR TYPICAL WALLS. (COMPRESSIVE STRENGTH = 2500 PSI AND 1800 PSI RESPECTIVELY. SITE TESTED MORTAR CUBES SHALL ACHIEVE A MINIMUM OF 80% OF THE DESIGN COMPRESSIVE STRENGTH).

M6 HAND MIXING MORTAR IS NOT ALLOWED.

M7 PIGMENTS WILL NOT BE ALLOWED IN MORTAR MIX.

M8 GROUT AND MORTAR SHALL BE FIELD TESTED AS DESCRIBED IN SECTION 3.7 (SMS) AND SPECIFICATIONS.

M9 GROUT FOR FILLED CELLS SHALL CONFORM TO ASTM C476, LATEST REVISION, AND SHALL HAVE A SLUMP OF BETWEEN 8" AND 10". PUMP 4'-0" MAXIMUM GROUT LIFTS WITH 60 MIN. DELAY BETWEEN LIFTS. GROUT COMPRESSIVE STRENGTH SHALL BE 3000 PSI AT 28 DAYS. ALL MASONRY BELOW SLAB OR GRADE SHALL BE SOLIDLY GROUTED. GROUT SHALL BE SAMPLED & TESTED ACCORDING TO ASTM C 1019 AT A FREQUENCY OF ONCE PER LIFT.

M10 SAMPLING AND TESTING WILL BE IN ACCORDANCE W/ SECTION 1.6 - TABLE 4 - LEVEL II QUALITY ASSURANCE (SMS).

M11 THE G.C. WILL PROVIDE CERTIFICATION FOR REINFORCING STEEL, JOINT REINFOREMENT, ANCHOR BOLTS, TIES, ANCHORS, METAL ACCESSORIES, AND CMU UNITS TO BE USED.

M12 REINFORCING BARS SHALL CONFORM TO ASTM A-615, GRADE 60.

M13 MATERIAL SHALL CONFORM TO THE FOLLOWING, EXCEPT AS NOTED:
PLATE AND BENT BAR ANCHORS: ASTM A572 GRADE 50.
SHEET METAL ANCHORS AND TIES: ASTM A366/A366M
WIRE MESH TIES: ASTM A 185 OR ASTM A 497.
WIRE TIES AND ANCHORS: ASTM A 82, & ASTM A167, TYPE 304.
ANCHOR BOLTS: ASTM A 307, GRADE A.

M14 REINFORCE JOINTS WITH LADDER-TYPE REINFORCEMENT CONFORMING TO ASTM A 951 AT 16" o.c. MEASURED VERTICALLY. LAP ALL JOINT REINFORCEMENT 6" MIN.

M15 REINFORCE MASONRY OPENINGS GREATER THAN 1'-0" WIDE, WITH HRIZ JOINT REINF PLACED IN (2) HORIZ JOINTS APPROXIMATELY 8" APART, IMMEDIATELY ABOVE THE LINTEL AND IMMEDIATELY BELOW THE SILL. EXTEND REINFORCING A MINIMUM OF 2'-0" BEYOND JAMBS OF THE OPENING EXCEPT AT CONTROL JOINTS. SEE PLAN FOR ADDITIONAL REQUIREMENTS.

M16 EXTEND ALL VERTICAL WALL REINFORCEMENT TO WITHIN 2" OF TOP OF WALL OR BEAM U.N.O. TERMINATE REINFORCING WITH STANDARD ACI 90 DEGREE HOOK IF ROOF JOISTS AND/OR TRUSSES BEAR ON TOP OF WALL AND THERE IS NO PARAPET. IF PARAPET EXISTS, HOOK IS NOT REQUIRED.

M17 MASONRY CONSTRUCTION JOINTS SHALL BE LOCATED AT ALL RETURNS AND SPACED NO GREATER THAN 24'-8". JOINTS SHALL ALSO BE PLACED AT A MINIMUM OF 2'-8" FROM OPENINGS. ALSO SEE DETAILS.

M18 JOINT FILLERS SHALL BE A PREMOLDED 3/8" JOINT FILLER.

M19 OPENINGS SHALL HAVE A MINIMUM OF ONE BLOCK CELL AT EACH JAMB GROUTED AND REINFORCED ALSO SEE DETAILS.

M20 PROVIDE PRECAST CONCRETE LINTEL @ ALL OPENINGS U.N.O. PROVIDE 8" BEARING EACH END, MIN. REINFORCE W/1-#5, EXTENDED 2'-0" EACH END. REFER TO DETAILS.

M21 AT FILLED CELLS, LAY UNITS WITH FULL BED JOINTS AROUND CELLS. USE PLAIN END TWO CELLED UNIT.

M22 UNLESS NOTED OTHERWISE BOND BEAMS ELEVATIONS ARE SHOWN ON ROOF PLANS..

M23 USE CORED HOLES W/ STEEL SLEEVES WHEN OPENINGS ARE REQUIRED FOR DRAIN PIPES. AVOID REINF. CELLS.

M24 WHERE CONCRETE BEAMS ARE INSTALLED IN CONCRETE BLOCK WALL, SUPPORT CONCRETE WITH 6" WIDE CONTINUOUS STRIPS OF POUR STOP MATERIAL (DUR-O-STOP OR EQUIV.) USE OF ROOFING FELT STRIPS OR ALUMINUM WILL NOT BE PERMITTED.

M25 IF TEMPRATURE FALLS BELOW 40 DEG F. OR EXCEEDS 100 DEG. F SPECIAL CONSTRUCTION MEASURES SHALL BE TAKEN AS PER FBC 2104.3 AND 2104.4.

M26 ALL MASONRY WALLS SHOWN ON THE STRUCTURAL DRAWINGS HAVE BEEN DESIGNED TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES IN THE FINAL CONSTRUCTED CONFIGURATION ONLY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ADEQUATELY BRACE THE WALLS FOR VERTICAL AND LATERAL LOADS THAT COULD POSSIBLY BE APPLIED PRIOR TO COMPLETION OF LATERAL SUPPORT BY CONNECTIONS AT FLOORS OR ROOF FRAMING LEVELS.

M27 GROUT PLACEMENT STOPPED FOR ONE HOUR OR MORE SHOULD BE STOPPED 1 1/2" BELOW THE TOP OF THE MASONRY UNIT TO PROVIDE A SHEAR KEY FOR SUBSEQUENT GROUTING.

M28 TYPICAL VERTICAL REINFORCING SIZE AND SPACING SHALL BE ABOVE AND BELOW ALL WALL OPENINGS.

METAL BUILDING GENERAL NOTES

MB1 SOUTHARD ENGINEERING, INC. IS ONLY RESPONSIBLE FOR FOUNDATION DESIGN. ALL OTHER STRUCTURAL COMPONENTS ARE TO BE DESIGNED BY THE METAL BUILDING DESIGN ENGINEER.

MB2 THE METAL BUILDING DESIGN ENGINEER SHALL CONSULT W/ THE M.E.P. ENGINEER TO DETERMINE THE APPROPRIATE DEAD LOADS FOR THE DESIGN OF THE METAL BUILDING SHELL.

MB3 FLORIDA PRODUCT APPROVAL NUMBERS SHALL BE SUBMITTED FOR THE ROOF PANELS & WALL PANELS.

MB4 SIGNED AND SEALED METAL BLDG. SHOP DRAWINGS SHALL BE SUBMITTED. SHOP DRAWINGS SHALL INCLUDE ALL COLUMN REACTION LOADS SO THAT THE FOOTINGS MAY BE VERIFIED.

MB5 ALL COLUMN ANCHOR BOLTS MUST HAVE A MINIMUM EDGE DISTANCE OF 12 INCHES.

MB6 MAX LATERAL STORY DRIFT SHALL BE LIMITED TO L/200.

TIMBER TRUSSES

TT1 A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.

TT2 SHOP DRAWING SUBMITTALS, INCLUDING, BUT NOT LIMITED TO, PLANS, DETAILS AND CALCULATIONS SHALL BE SUBMITTED TO ARCHITECT FOR REVIEW PRIOR TO FABRICATION. CALCULATIONS SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS PROJECT LOCATION. "SOUTHARD ENGINEERING, INC." WILL REQUIRE THAT THE ENGINEERED DRAWINGS FOR THE WOOD TRUSSES BE REVIEWED FOR COMPATIBILITY WITH THE DESIGN INTENT OF THE STRUCTURE PRIOR TO FABRICATION. ANY AND ALL COSTS ASSOCIATED WITH FABRICATING TRUSSES FROM SUBMITTALS NOT BEARING OUR SHOP DRAWING STAMP AND APPROVAL WILL BE THE SOLE RESPONSIBILITY OF THE G.C.

TT3 ALL TRUSSES SHALL BE DESIGNED BY A SPECIALTY ENGINEER WITH A MINIUM OF 5 YEARS EXPERIENCE IN SIMILAR TYPE STRUCTURES.

TT4 ALL TRUSSES SHALL BE TEMPORARILY AND PERMANENTLY BRACED AS REQUIRED BY TRUSS MANUFACTURER.

TT5 USE SOUTHERN YELLOW PINE FOR ALL TRUSS MEMBERS.

TT6 THE ROOF PLAN SHOWN HEREIN IS A GRAPHICAL REPRESENTATION ONLY. REFER TO TRUSS SUBMITTALS FOR ACTUAL LAYOUT, TRUSS PROFILES AND HOLD-DOWN REQUIREMENTS AT ALL TRUSS SUPPORTS.

TT7 ALL TRUSS TO TRUSS CONNECTIONS SHALL BE DESIGNED AND PROVIDED BY TRUSS MANUFACTURER. ALL TRUSS BEARING CONNECTIONS (AT SEATS) TO SUPPORTS SHALL BE DESIGNED AND PROVIDED BY TRUSS MANUFACTURER TO RESIST TRUSS UPLIFT REACTIONS.

TT8 TRUSSES SHALL BE SHOP FABRICATED, INCLUDING ANY FIELD SPLICE CONNECTION COMPONENTS AND SHIPPED TO SITE IN MAXIMUM LENGTHS AND HEIGHTS. FIELD FABRICATION OF TRUSSES WILL NOT BE PERMITTED.

TT9 ALL TOP AND BOTTOM CHORDS SHALL BE PRESSURE TREATED UNLESS NOTED OTHERWISE.

TT10 REVIEW ARCHITRECTURAL FLOOR PLANS FOR OPERABLE WALLS THAT ARE SUSPENDED FROM THE TRUSSES. ANALYSIS FOR CLOSED, PARTIDALLY OPEN AND OPEN POSITION LOAD CURVES IS REQ'D. REFER TO WALL MANUFACTURER FOR SPECIAL DEFLECTION CRITERIA.

TT11 REVIEW ARCHITECTURAL REFLECTED CEILING PLANS AND SECTIONS FOR SPECIAL CEILING CONDITIONS, INCLUDING CEILING SLOPES, TROFFERS, COFFERS, TRAYS, STEPS AND OTHER SPECIAL FEATURES.

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S.1